

## **BRAMPTON MATERIALS ENGINEERING**

Construction Materials Inspection & Testing, Geotechnical Engineering, Building Science

Link Aluminum Railing & Guard LTD 589 Miiddlefield Rd, Scarborough, ON M1V 2Z1

October 2, 2024										
Project Manager										
125 Euphemia Street South, Sarnia, ON										
Guardrail Load Testing										
Balcony Guardrails in Suite 404 Our Reference: 4162-TR-GR										
<ol> <li>Ontario Building Code 2012, Articles 4.1.5.14 and 4.1.4.16.</li> <li>Standard Test Method for Performance of Permanent Metal Railing Systems and Rails for Buildings (ASTM E935n-13 e1)</li> </ol>										

3. CSA A500-16 Building Guards (Table 5.1, Test References A and D)

#### 1. Introduction

As requested, Brampton Materials Engineering (BME) visited the above-noted site on September 26, 2024, to perform load testing of balcony guardrails in accordance with the referenced standards and procedures discussed below.

#### 2. Building Codes and Standards

#### 2.1 Ontario Building Code 2012, Article 4.1.5.14, Live Loads on Guards

- 2.1.1 The minimum specified horizontal load applied inward or outward at the top of every required guard shall be,
  - (a) 3.0 kN/m for means of egress in grandstands, stadia, bleachers and arenas,
  - (b) a concentrated load of 1.0 kN applied at any point for access ways to equipment platforms, contiguous stairs and similar areas where the gathering of many people is improbable,
  - (c) 0.75 kN/m or a concentrated load of 1.0 kN applied at any point, whichever governs for locations other than those described in subclauses (a) and (b).
  - (d) Individual elements within the guard, including solid panels and pickets, shall be designed for a load of 0.5 kN applied over an area of 100 mm by 100 mm located at any point in the element or elements so as to produce the most critical effect.
- 2.1.2 The loads required in Sentence (2) need not be considered to act simultaneously with the loads provided for in Sentences (1) and (4),
- 2.1.3 The minimum specified load applied vertically at the top of every required guard shall be 1.5 kN/m and need not be considered to act simultaneously with the horizontal load provided for in Sentence (1).

- 2.2 Ontario Building Code 2012, Subsection 4.1.7, Specified Wind Load
- 2.3 Ontario Building Code 2012, Table 4.1.3.2.A, Load Combinations for Ultimate Limit States: Column 1, Case 2 (1.5 L+0.4W) and Case 4 (1.4W+0.5L)

#### 2.4 ASTM E985-00 (2006, withdrawn 2015) Deflection Criteria:

- 2.4.1 The maximum allowable deflection at the required test load, measured at the top of the rail at the point of load application and from the position of the rail after release of the preload, shall not be more than the following:
  - 2.4.1.1 Horizontal deflection at the top of the post = rail height(h)/12
  - 2.4.1.2 Horizontal deflection at mid span = (h/24) + span(I)/96 or 57 mm
  - 2.4.1.3 Vertical deflection at the mid span = Span(I)/96
  - 2.4.1.4 Residual deflection at the released test load shall not exceed 20 % of the deflection permitted or 13 mm whichever is smaller

#### 2.5 CSA A500-16, Article 5.5.1.6, Deflection Criteria

The difference between the deflection after the peak service load (225 lbs.) has been released and the deflection at "zero" load shall be less than 5 mm and shall not increase with repeated application of the peak service load.

#### 3. Test Equipment and Devices

- 3.1 Loading Equipment: A Calibrated hydraulic pump with a digital pressure gauge and hydraulic cylinders,
- 3.2 Deflection Measuring Device: A digital laser distance meter with tripod setup
- 3.3 100 mm x 100 mm bearing plate to test the infill.

#### 4. Test Procedure

#### 4.1 Loading Procedure

- 4.1.1. The guardrail section was preloaded to 50% of the peak service load to bring all members in full bearings and then the load was released to zero,
- 4.1.2. Initial deflection (displacement) was noted prior to application of test loads,
- 4.1.3. A horizontal point load of 225 lbs. was applied at top of the intermediate post at 1070 mm above finish floor level (FFL). Residual displacement was noted and checked for compliance with Section 2.5 requirement,
- 4.1.4. Various guardrail elements were loaded individually with 15% load increment and displacement noted at each loading,
- 4.1.5. Displacement at ultimate load was noted and checked for compliance with Section 2.4 requirements.
- 4.1.6. The ultimate load was held for 5 minutes and then released to zero,
- 4.1.7. Residual displacement noted and checked for compliance with Section 2.5 requirement.

#### 4.2 Loading

A horizontal point load of 225 lbs. was applied at the top of an intermediate post (Section L-L in Figure 1a). The load was then increased to 430 lbs. (ultimate load) without failure. The maximum and residual displacements were noted and checked for compliance with the deflection criteria of Sections 2.4 and 2.5.

A horizontal point load of 225 lbs. was applied at the top of an end post (Section K-K in Figure 1a). The load was then increased to 365 lbs. (ultimate load) without failure. The maximum and residual displacements were noted and checked for compliance with the deflection criteria of Sections 2.4 and 2.5.

A horizontal point load of 225 lbs. was applied at the handrail mid-section (Section L-R in Figure 1a). The load was then increased to 430 lbs. (ultimate load) without failure. The maximum and residual displacements were noted and checked for compliance with the deflection criteria of Sections 2.4 and 2.5.

A horizontal outward load of 115 lbs. was applied at the mid-section of infill picket panel (Section L-R in Figure 1a). The load was then increased to 175 lbs. without failure. The maximum and residual displacements were noted and checked for compliance with the deflection criteria of Sections 2.4 and 2.5.

A horizontal outward load of 25 lbs. was applied at the mid-section of two pickets (Section M-K in Figure 1a) to check compliance with the opening criteria of Article 9.8.8.5 of OBC 2012.

#### 5. Test Results

The test results are shown in Tables 1A, 1B, 1C, and 1D and provided in Appendix A. Several photographs were taken during the testing. Select photographs are provided in Appendix B, while the remaining are kept on file for future reference.

#### 6. Conclusions

The guardrail system meets the requirements of OBC 2012, Article 4.1.5.14 for live load with safety factor of 1.91 for guardrail and 1.52 for the picket infill panels. The guardrail system meets the requirements of OBC 2012, Article 9.8.8.5 for opening size. The guardrail system also meets the requirements of CSA A500-16, Table 5.1, Test References A and D.

We trust this report meets your requirements. If there are any questions or further consultation is required, please feel free to contact the undersigned.

Respectfully Submitted,

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Haroon Raza, M. Eng., P. Eng. Senior Materials Engineer

Encl: Appendix A - Tables and Figures Appendix B - Photographs





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**APPENDIX A** 

### Table 1A: Summary of Guardrails Load Testing (OBC 2012)

Client:Link Aluminum Railing & Guard LTD

Project: 125 Euphemia Street South, Sarnia, ON

Date: 02-Oct-24

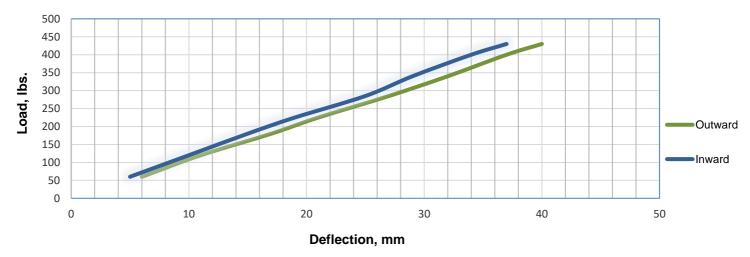
Project No.: 4162-TR-GR

Location: Suite 404

Date Tested: 26-Sep-24

					Lo	ad				(r						
					Requ	uired				un Tr						
Guardrail Member	Type of Load	Section	Span (mm)	Sectional Height (mm)	Concentrated Load (Ibs.)	Distributed Load (lb / ft.)	•	Actual Load Applied (lbs.)	Actual Load Applied (kN)	Allowable Maximum Initial Displacement (mm)	Actual Initial Displacement (mm)	Elapsed Time (min.)	Allowable Residual Displacement (mm)	Actual Residual Displacement (mm)	Remarks	
					225		225	225	1.00		21.0		< 5	2		
	ard				0		0	0	0.00		2.0	5	_			
	itwa	Horizontal - Outward				60		60	60	0.27		6.0				
					120		120	120	0.54		11.0				<b>Pass</b> No structural or	
					180 225		180 225	180 225	0.80		17.0 21.0					
, T	ont				225		225	225	1.00		27.0			3		
soc	oriz				340		340	340	1.52		32.0					
te F	Я				400		400	400	1.78		37.0					
dia		L-L	1090	1100	430		430	430	1.92	92	40.0	5				
Intermediate Post		1			60		60	60	0.27		5.0		13		mechanical	
nter	Ird				120		120	120	0.54		10.0				connection failure	
=	Inward				180		180	180	0.80		15.0				was observed	
					225		225	225	1.00		19.0				during the test	
	ntal				285		285	285	1.27		25.0			3		
	izo				340		340	340	1.52		29.0		'			
	Horizontal				400		400	400	1.78		34.0					
	-				430		430	430	1.92		37.0	5				

Load - Deflection Chart



## Table 1B: Summary of Guardrails Load Testing (OBC 2012)

Client: Link Aluminum Railing & Guard LTD

Project: 125 Euphemia Street South, Sarnia, ON

Date: 02-Oct-24

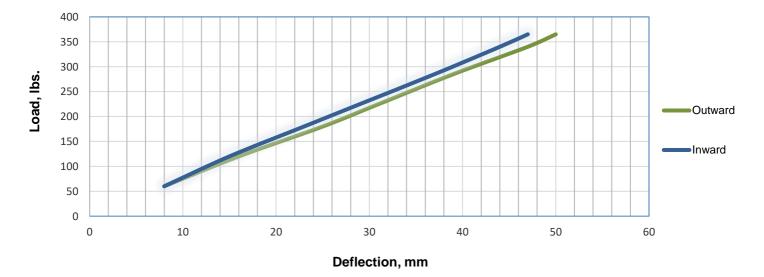
Project No.: 4162-TR-GR

Location: Suite 404

Date Tested: 26-Sep-24

						ad				tial							
					Requ	uired	_			in							
Guardrail Member	Type of Load	Section	Span (mm)	Sectional Height (mm)	Concentrated Load (Ibs.)	Distributed Load (lb / ft.)		Actual Load Applied (lbs.)	Actual Load Applied (kN)	Allowable Maximum Initial Displacement (mm)	Actual Initial Displacement (mm)	Elapsed Time (min.)	Allowable Residual Displacement (mm)	Actual Residual	Remarks		
	ard				60		60	60	0.27	e .	8.0						
	twa	tal - Outward					120 180	4	120 180	120 180	0.54 0.80		16.0 25.0				
						<b>225</b> 225 225 1.00 <b>31.0</b>											
	tal -							285		285	285	1.27		39.0			3
	cont				340		340	340	1.52		47.0				Pass		
	Horizontal				365		365	365	1.63		50.0				No structural or		
End Post	Ĭ										5			mechanical			
Ρ	-	K-K		1100	60		60	60	0.27	92	8.0		13		connection failure		
En	Inward				120		120	120	0.54		15.0				was observed		
					180		180	180	0.80		23.0				during the test		
	al -				225		225	225	1.00		29.0			3	dannig the tool		
	ont				285		285	285	1.27		37.0			5			
	Horizontal				340		340	340	1.52		44.0						
	Но				365		365	365	1.63		47.0						
												5					

#### **Load - Deflection Chart**



## Table 1C: Summary of Guardrails Load Testing (OBC 2012)

Client: Link Aluminum Railing & Guard LTD

Location: Suite 404

Project: 125 Euphemia Street South, Sarnia, ON

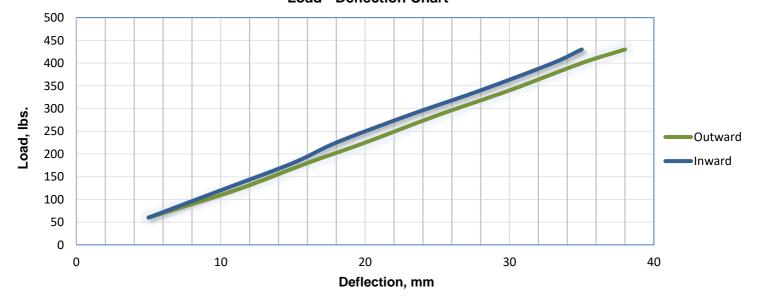
Project No.: 4162-TR-GR

Date: 02-Oct-24

Date Tested: 26-Sep-24

_		_		_	_	_		_	_	_			_				_	_	_	-
Guardrail Member	Type of Load	Section	Span (mm)	Sectional	Height (mm) Concentrated Load	concentrated (lbs.)	ted / # /	Load (ID / TL.) Gauge		Actual Load Applied (lbs.)		Allowable Maximum Initial	Uisplacement (mm)	Actual Initial Displacement (mm)	Elapsed Time (min.)	Allowable Residual	Displacement (mm)	Actual Residual	Displacement (mm)	Remarks
Handrail Mid-Section	Horizontal - Inward Horizontal - Outward	L-R	1090	110		60         120         180         225         340         400         430         60         120         180         225         340         400         180         225         340         400         430         400         430		60 12 18 22 28 34 40 43 60 12 18 22 28 34 40 43	0 0 5 5 0 0 0 0 0 0 5 0 0 0 0 0 0 0 0 0 0 0 0 0	60 120 225 285 340 400 430 60 120 180 225 285 340 400 430	0.27 0.54 0.80 1.00 1.27 1.52 1.78 1.92 0.27 0.54 0.80 1.00 1.27 1.52 1.78 1.92	57		5.0 11.0 20.0 25.0 30.0 35.0 38.0 5.0 10.0 15.0 18.0 23.0 28.0 33.0 35.0	5	1	1	3		<b>Pass</b> No structural or mechanical connection failure was observed during the test

## Load - Deflection Chart



## Table 1D: Summary of Guardrails Load Testing (OBC 2012)

Client: Link Aluminum Railing & Guard LTD

Project: 125 Euphemia Street South, Sarnia, ON

Date: 02-Oct-24

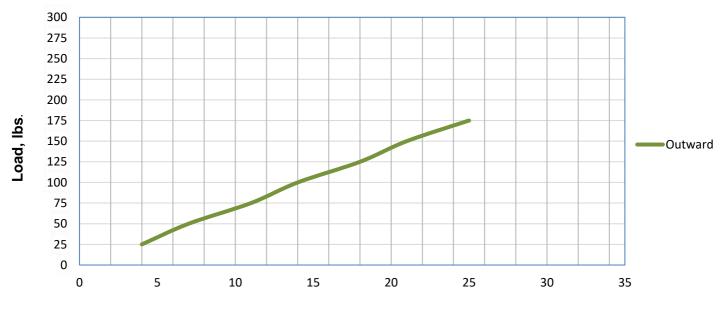
Project No.: 4162-TR-GR

Location: Suite 404

Date Tested: 26-Sep-24

Guardrail Member	Type of Load	Section	Span (mm)	Sectional Height (mm)	Requ peoq	Distributed Distributed Data Distributed D	Gauge Reading (psi)	Actual Load Applied (Ibs.)	Actual Load Applied (kN)	Allowable Maximum Initial Displacement (mm)	Actual Initial Displacement (mm)		Allowable Residual Displacement (mm)	Actual Residual Displacement (mm)	Remarks
					25		25	25	0.11		4.0				
<del>.</del>	ard				50 75 100		50	50	0.22		7.0				Pass
Jan	Outward						75	75	0.33		11.0				No structural or
ket I		L-R	1090	1100			100	100	0.45	34	14.0		7	5	mechanical
Infill Picket Panel Horizontal - Outwal	ntal	L-R	1090	1100	125		125	125	0.56	34	18.0			5	connection failure
	rizol				150		150	150	0.67		21.0				was observed during
<u> </u>	Hor				175		175	175	0.78		25.0				the test
												5			

#### **Load-Deflection Chart**



Deflection, mm

125 Euphemia Street South, Sarnia, ON

September 26, 2024

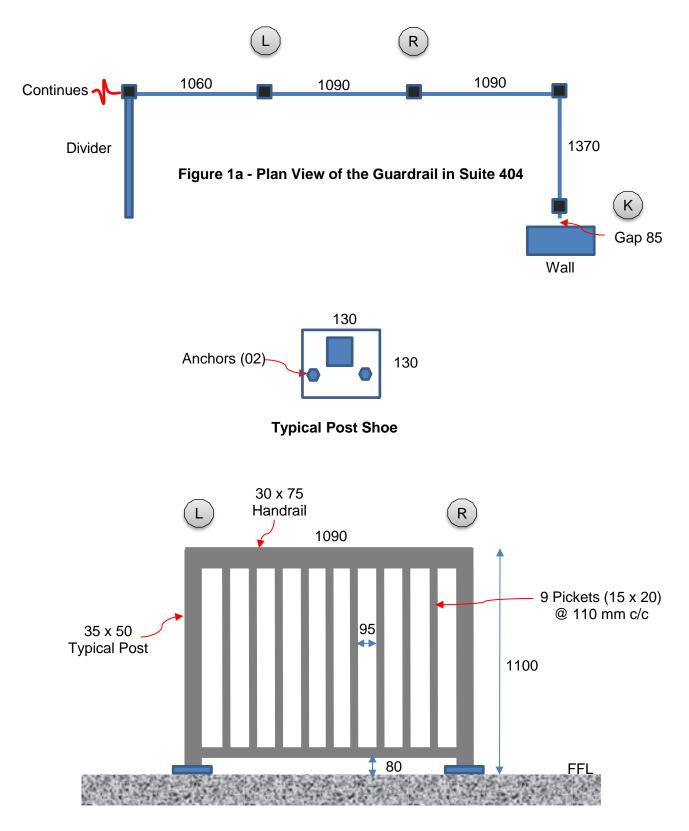


Figure 1b - Elevation

(All dimensions in millimeters, NTS)

# **APPENDIX B**



Photograph 1: General overview of the building



Photograph 2: General overview of the guardrail



Photograph 3: Typical guardrail shoe detail



Photograph 4: Showing outward load application at the top of an intermediate post



Photograph 5: Showing inward load application at the top of an intermediate post



Photograph 6: Showing outward load application at the top of an end-post

BME-GEN-0147H-05-22



Photograph 7: Showing inward load application at the top of an end-post



Photograph 8: Showing outward load application at the handrail mid-section



Photograph 9: Showing inward load application at the handrail mid-section



Photograph 10: Showing outward load application at the infill picket panel mid-section





Photograph 11: Checking picket gap by pushing a conical aluminum cone between two pickets